



CHALLENGES AND FUTURE POTENTIAL APPLICATIONS OF CFD IN RESTORATION HYDRAULICS

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INTRODUCTION

This paper focuses on emerging hydrodynamics and sediment transport questions in restoration infrastructure that can potentially be addressed by CFD simulations. These include:

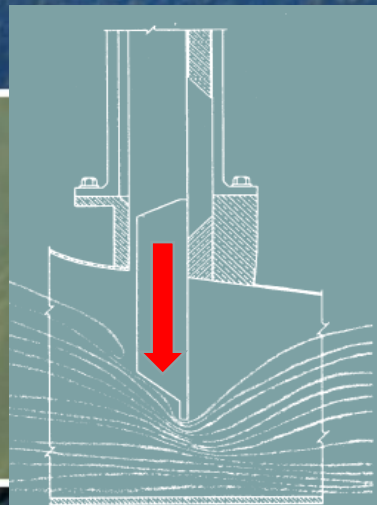
- Hydraulic downpull on spillway gates
- Depth and extent of scour downstream of hydraulic structures
- Role of pressure fluctuations in structure failure
- 3D simulations of vegetated flows

HYDRAULIC DOWNPULL

The hydraulic downpull is a downward hydrodynamic force on high head spillways gates that is caused by a localized flow acceleration, and associated pressure drop, at or near the gates lips.

Its accurate estimate is important for a proper design of the lifting (hoist) mechanism of the gates. A properly designed hoist mechanism can withstand the hydraulic downpull, the weight of the gate, and the weight of the fluid above it. Previous studies have shown the hydraulic downpull is affected by

- Geometry of the gates
- Flow and velocity field underneath of the gates



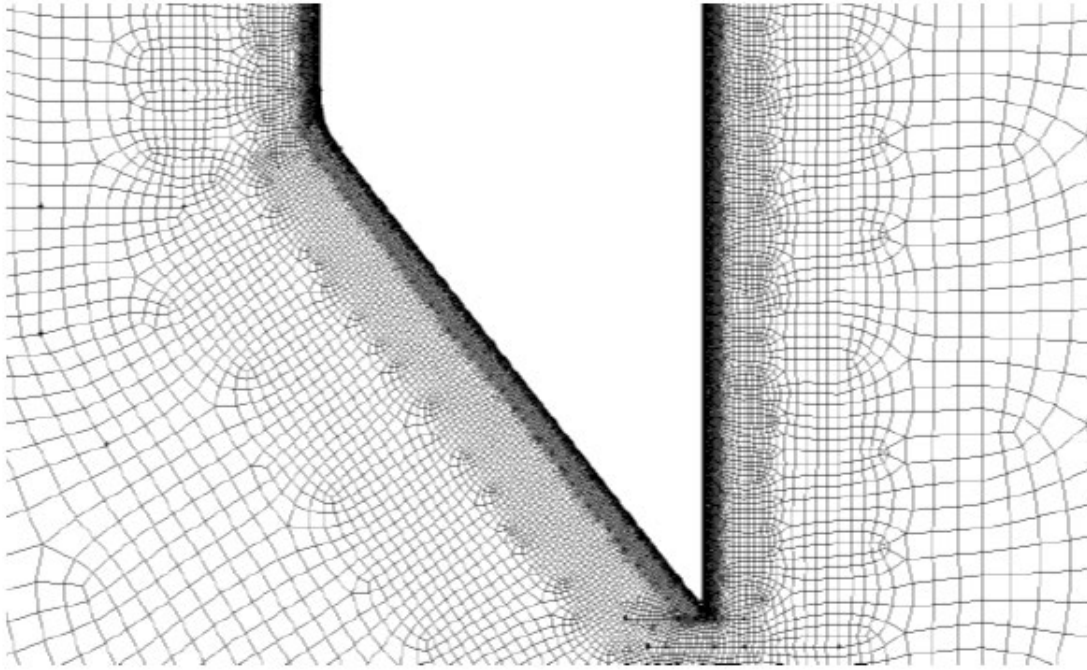


Figure 3.4 Mesh around the gate

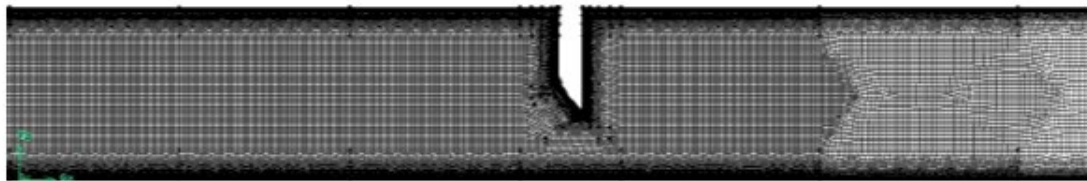


Figure 3.5 Mesh around gate and parts of upstream and downstream

EXAMPLE OF PAST CFD APPLICATIONS IN ESTIMATING HYDRAULIC DOWNPULL

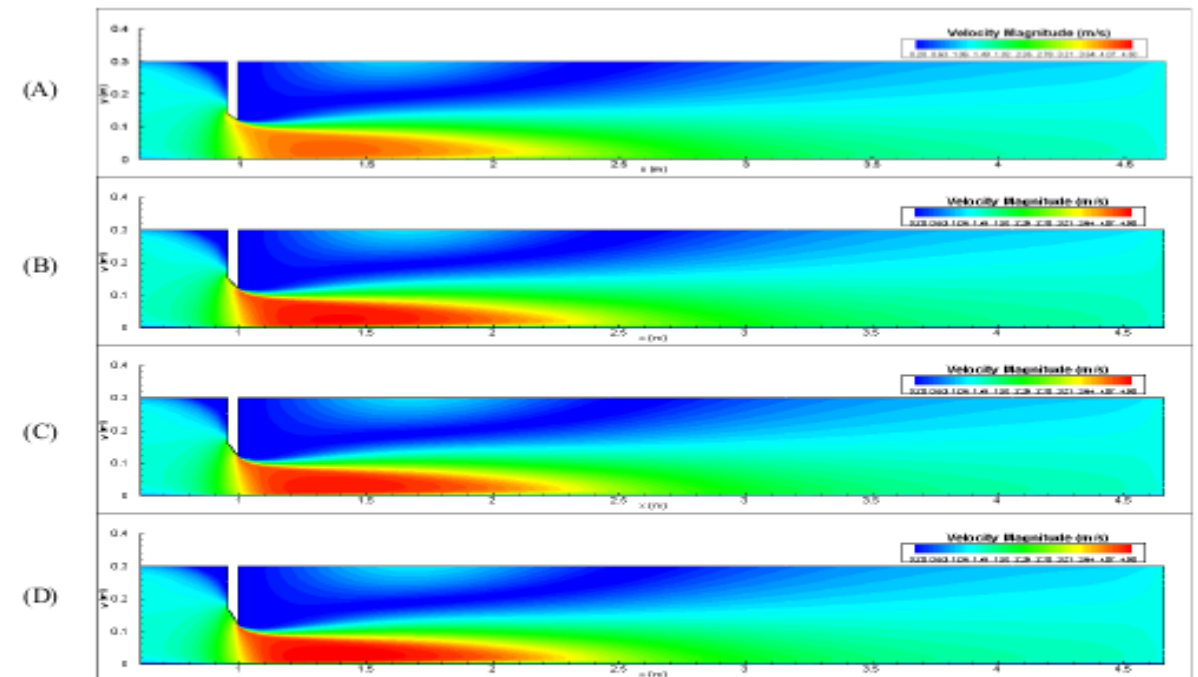


Figure 3.22 Velocity magnitude distribution for (A) $\theta=26.5^\circ$, $y=0.4$, $Q=0.0947 \text{ m}^3/\text{s}$, (B) $\theta=36.7^\circ$, $y=0.4$, $Q=0.0997 \text{ m}^3/\text{s}$, (C) $\theta=44.7^\circ$, $y=0.4$, $Q=0.0955 \text{ m}^3/\text{s}$, (D) $\theta=51.6^\circ$, $y=0.4$, $Q=0.0953 \text{ m}^3/\text{s}$

$$D_P = \gamma_w \cdot K_L \cdot A \cdot H$$

where

D_P = Downpull force on the gate

γ_w = Specific weight of water

K_L = Downpull force coefficient

A = Cross-sectional area of

H = Operating head on the

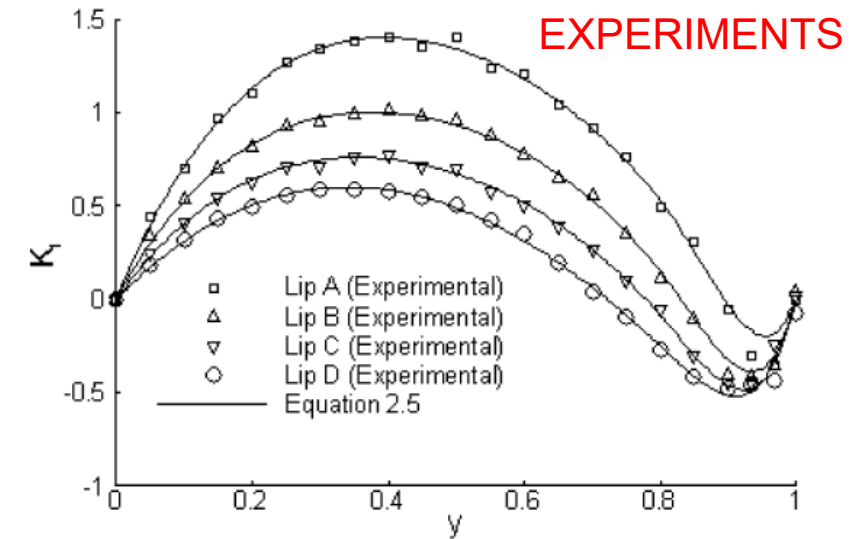
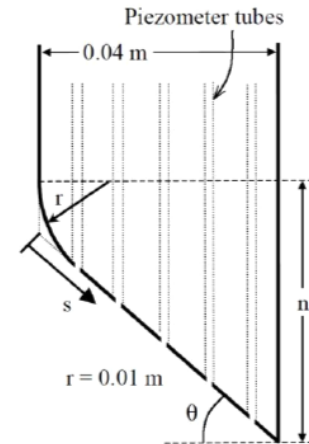


Figure 2.14 Downpull coefficient as a function of gate opening and gate lip angle (Aydin et al., 2003)

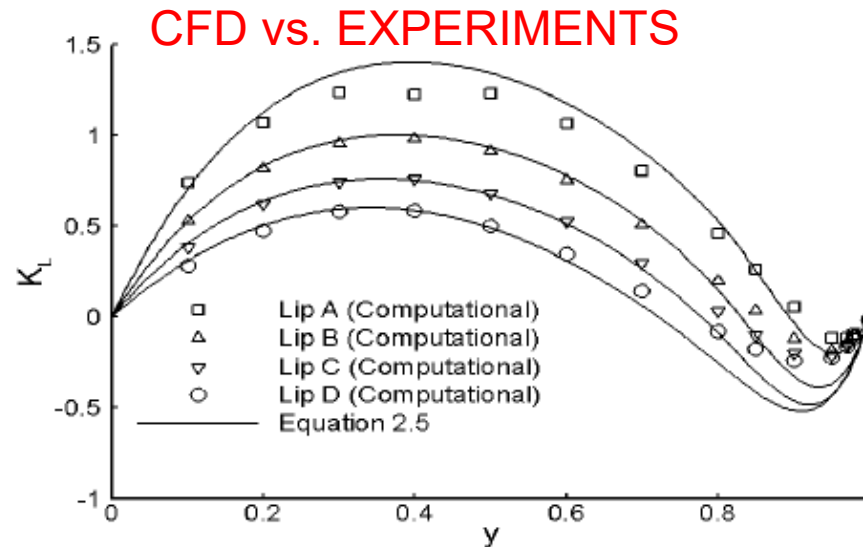


Figure 3.31 Downpull coefficient as a function of the gate lip angle and the gate opening (Computational and experimental comparison)

Table 2.1 Gate lip angles

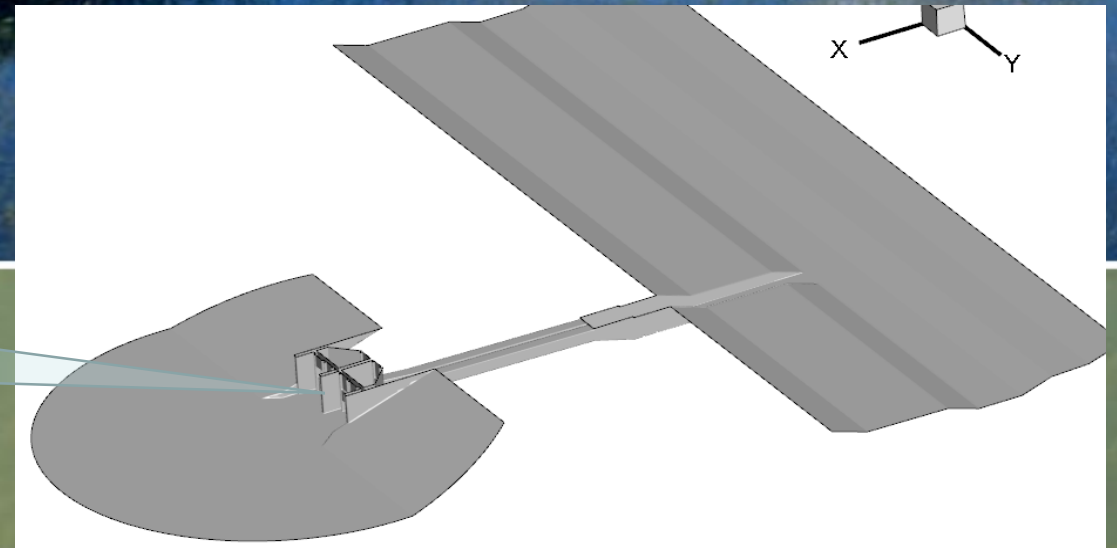
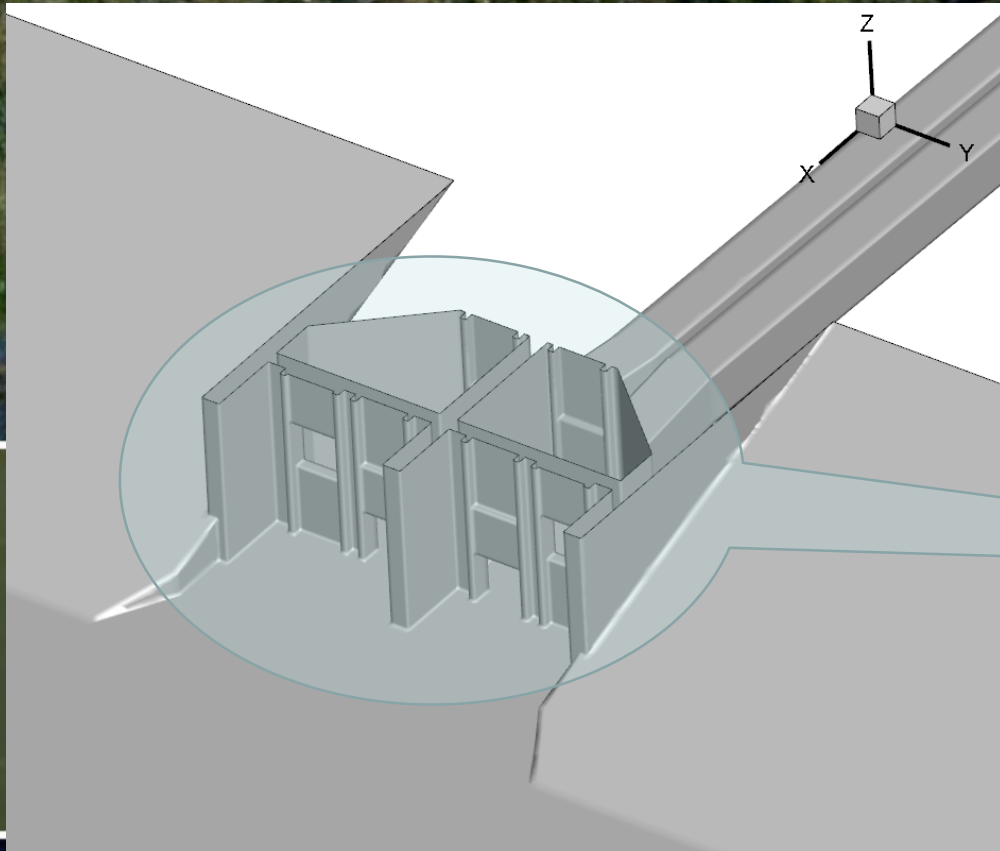
Lip Symbol	n (cm)	Lip angle, θ (degrees)
A	2	26.5
B	3	36.7
C	4	44.7
D	5	51.6

Uysal (2014)

Potential Application: HYDRAULIC DOWNPULL @ C43

Reservoir outflow structures

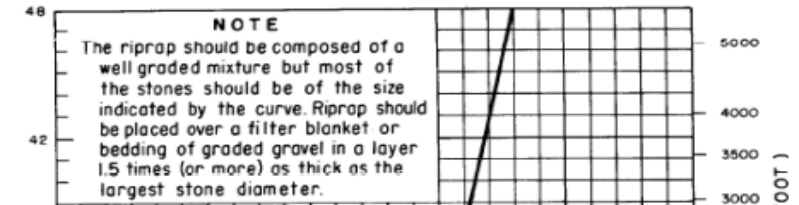
Outflow structures (S471 & S473)
Estimated Downpull forces > 20,000 lbf



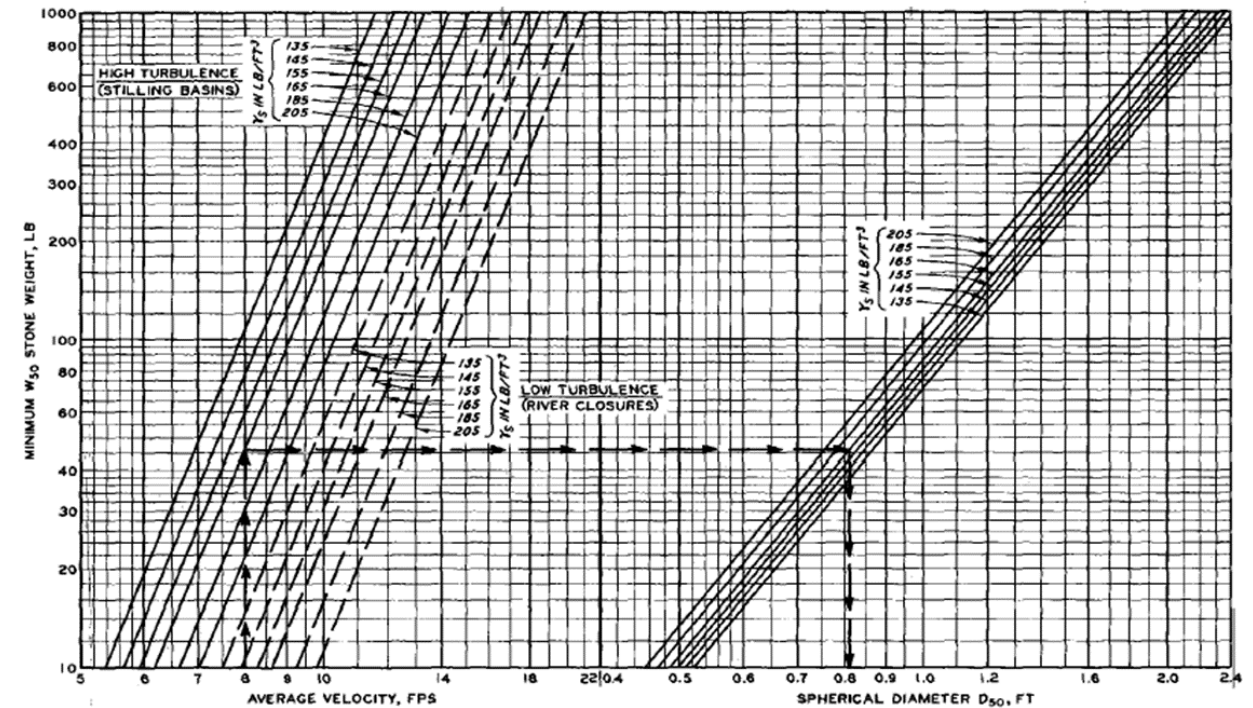
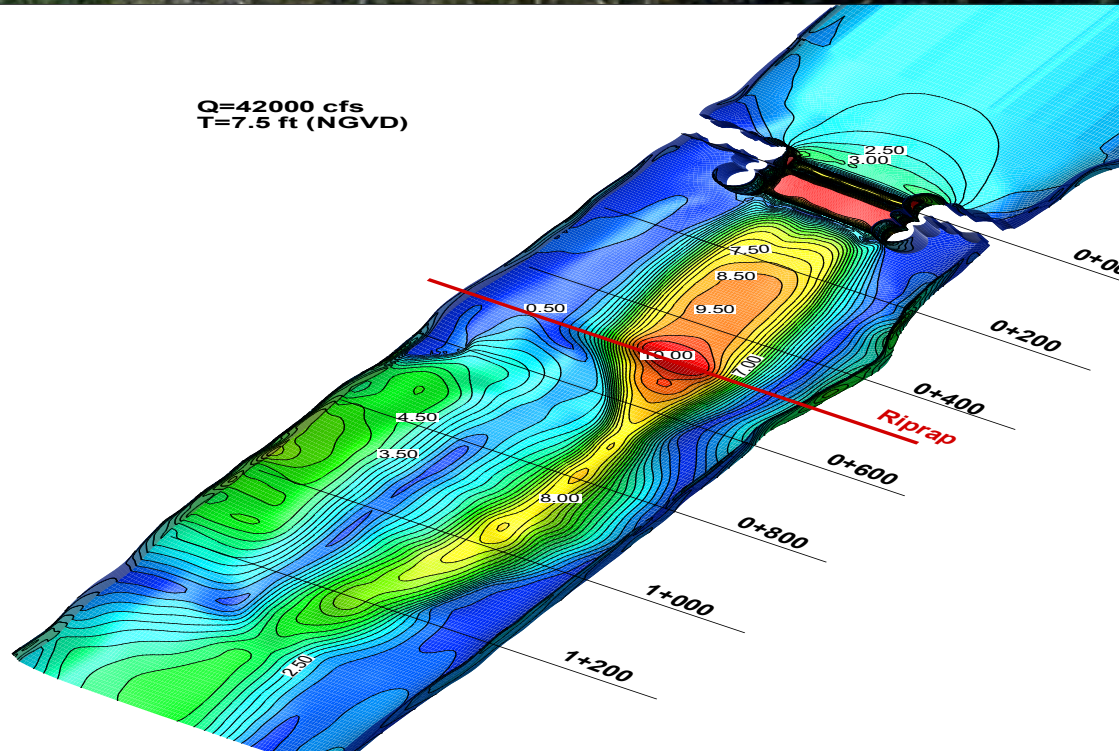
SCOUR DOWNSTREAM OF STRUCTURES

- Currently we use estimated velocities from CFD simulation to infer scour potential
- Depth and extent of scour downstream of hydraulic structures is not directly estimated

SIZE OF RIPRAP TO BE USED DOWNSTREAM FROM STILLING BASINS



Q=42000 cfs
T=7.5 ft (NGVD)



BASIC EQUATIONS

$$V = C \left[2g \left(\frac{\gamma_s - \gamma_w}{\gamma_w} \right) \right]^{1/2} (D_{50})^{1/2}$$

$$D_{50} = \left(\frac{6W_{50}}{\pi \gamma_s} \right)^{1/3}$$

WHERE:

- V = VELOCITY, FPS
- γ_s = SPECIFIC STONE WEIGHT, LB/FT³
- γ_w = SPECIFIC WEIGHT OF WATER, 62.5 LB/FT³
- W_{50} = WEIGHT OF STONE, SUBSCRIPT DENOTES PERCENT OF TOTAL WEIGHT OF MATERIAL CONTAINING STONE OF LESS WEIGHT.
- D_{50} = SPHERICAL DIAMETER OF STONE HAVING THE SAME WEIGHT AS W_{50}
- C = ISBASH CONSTANT (0.86 FOR HIGH TURBULENCE LEVEL FLOW AND 1.20 FOR LOW TURBULENCE LEVEL FLOW)
- g = ACCELERATION OF GRAVITY, FT/SEC²

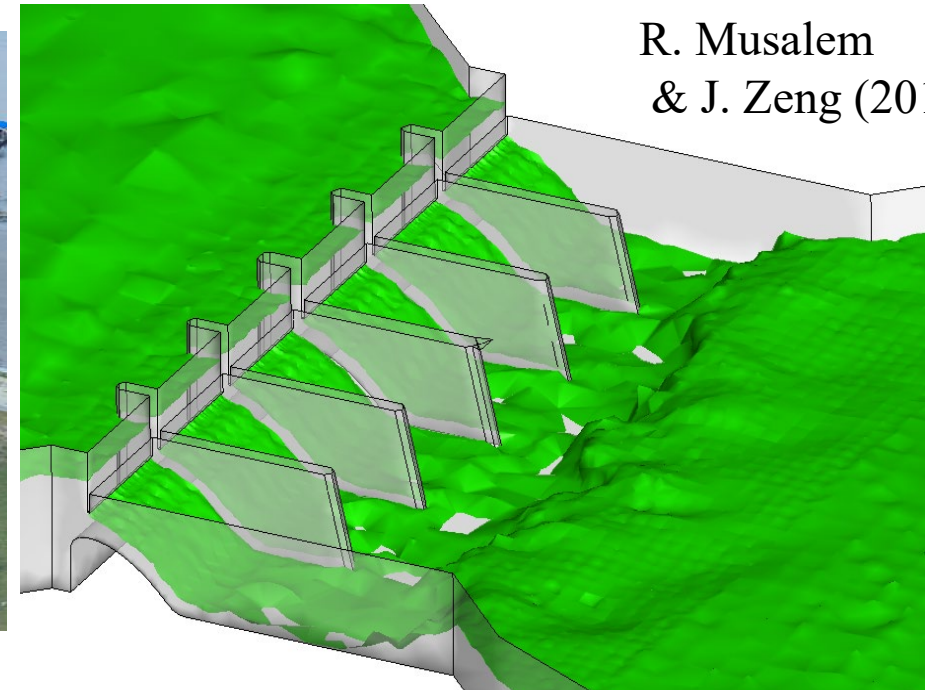
USACE
STONE STABILITY
VELOCITY VS STONE DIAMETER

HYDRAULIC DESIGN CHART 712-1
(SHEET 1 OF 2)

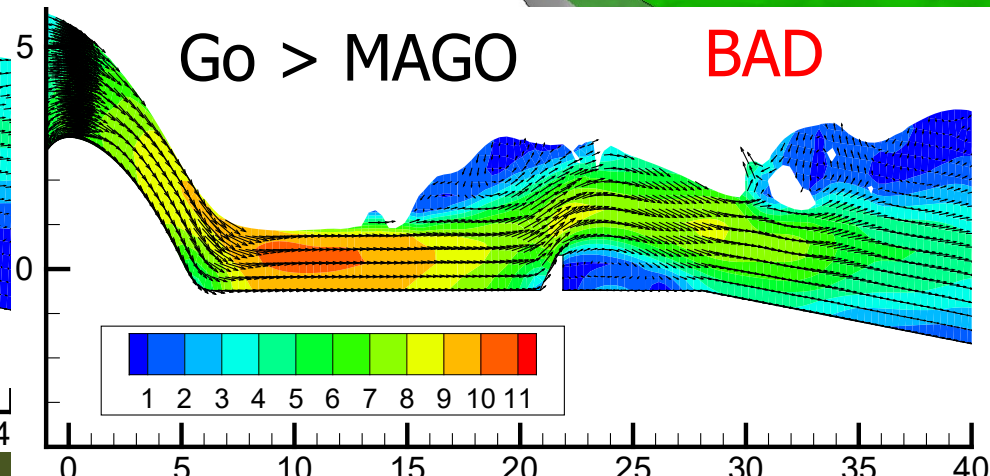
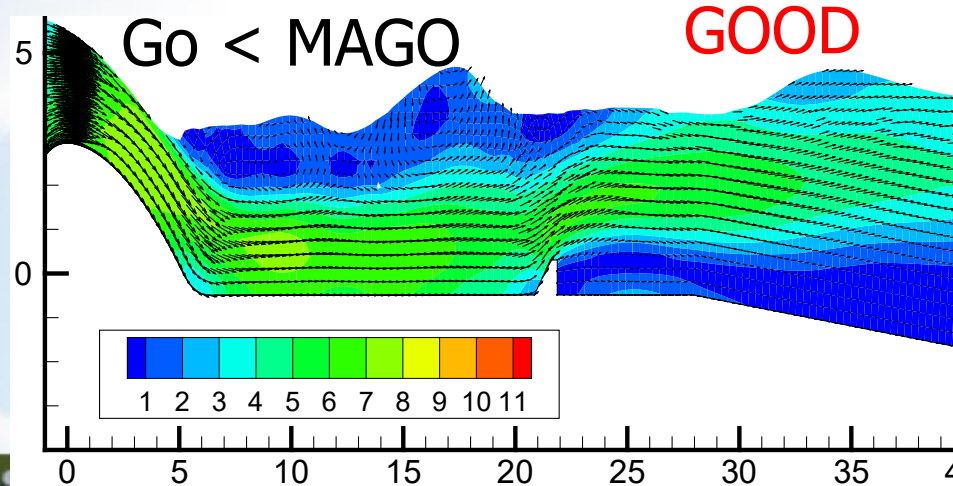
REV 8-58, 9-70

WES 6-57

Another Erosion Potential Check: Location of the Hydraulic Jump

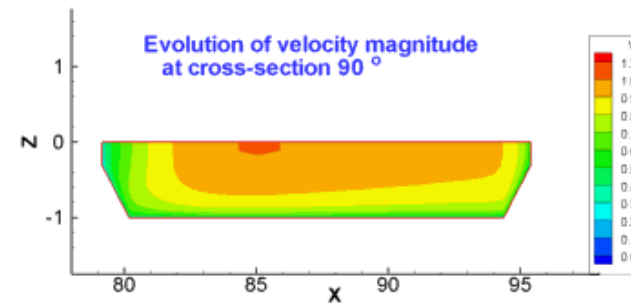
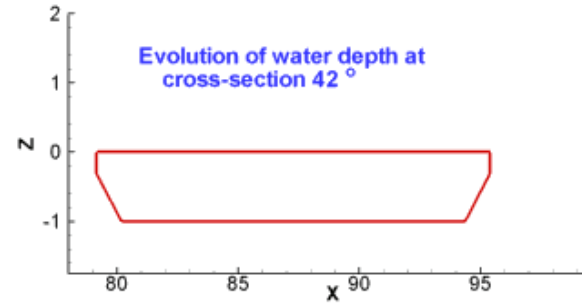
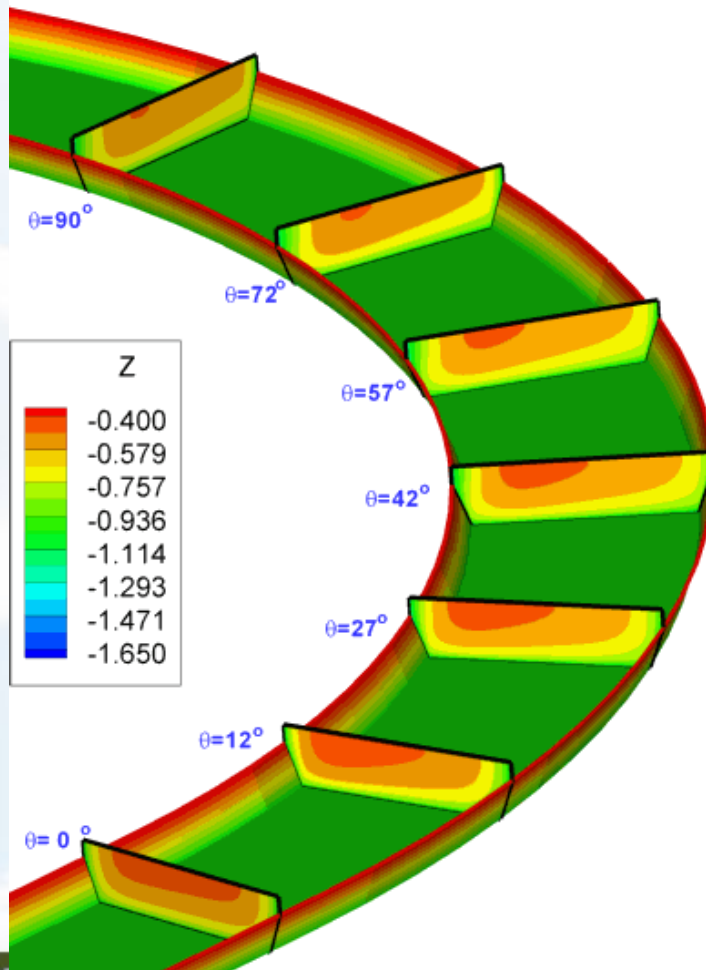


R. Musalem
& J. Zeng (2012)



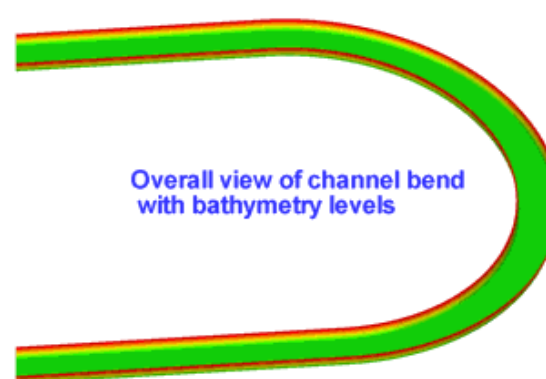
Application of CFD to estimate sedimentation in channel Bend

Evolution of channel bend bathymetry and velocity magnitude from flat bed initial configuration to equilibrium



Scour takes place on the outer bank side

Deposition is observed near inner bank



Model Verification and Analysis

Water depth changes at representative sections

Advection-Dispersion
Eqn. and Van Rijn (1987)
Model are used
for modeling
sediment transport

$$C_{b*} = 0.015 \frac{d_{50} T^{1.5}}{a D_*^{0.5}} \quad \text{Equilibrium Con.}$$

Equilibrium bed load rate

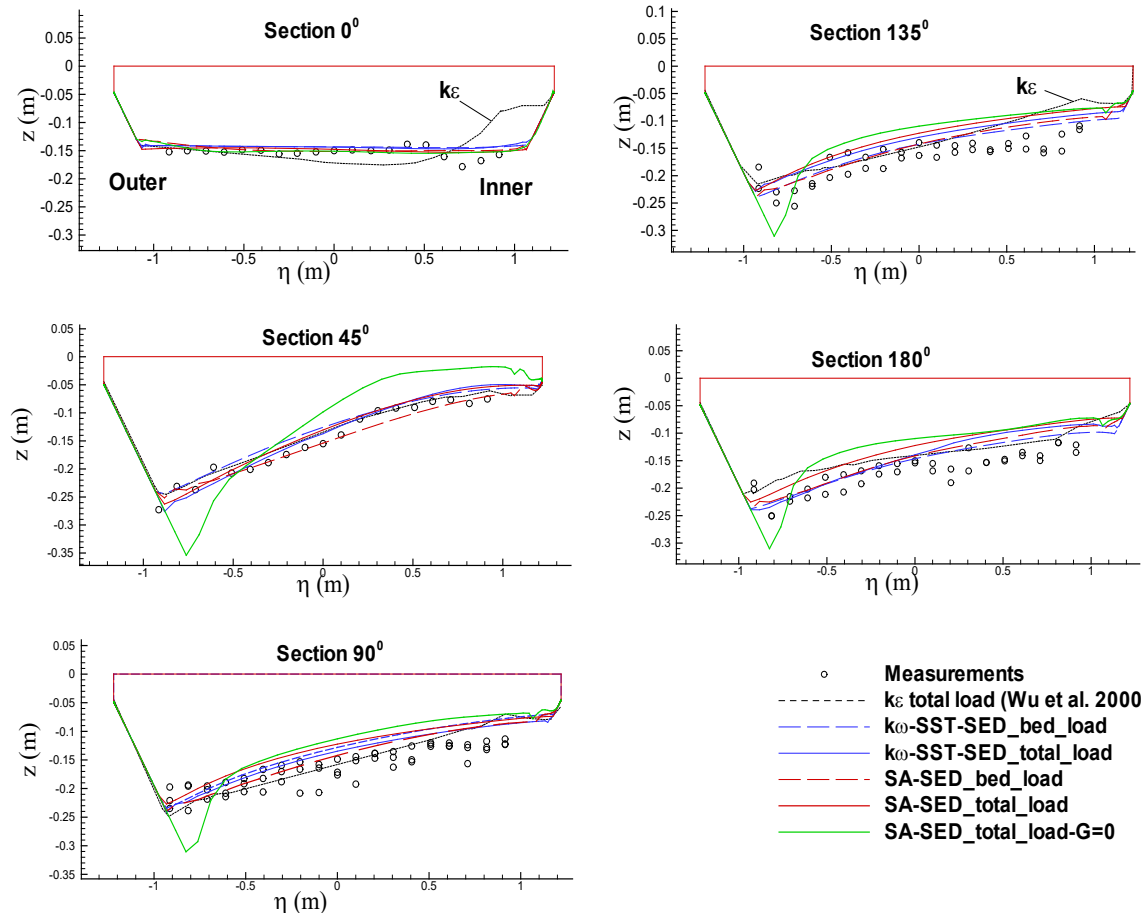
$$Q_{b*} = 0.053 (Rg)^{0.5} (d_{50}^{15} T^{2.1}) / (D_*^{0.3})$$

Non Dim. Excess shear stress

$$T = [\tau'_* - \tau_{cr}] / \tau_{cr} = [(u'_*)^2 - (u_{*cr})^2] / (u_{*cr})^2$$

$R = \rho_s / \rho - 1$ is the reduced gravity,

the non-dimensional particle-size diameter is $D_* = d_{50} [Rg/v^2]^{1/3}$,



For $\theta > 20^\circ$
triangular
Shape

Short sand
bar observed
near inner
bank

Maximum scour
 $\theta \sim 50^\circ$

- The newly developed model predicts well the change of water depth in various sections along the channel.
- Sparlat-Allmaras (SA) model with total load give a slightly better predictions compared with other simulation results.
- Without considering the bed slope effect, the outer bank scour inner bank deposition are over-predicted.

Instantaneous Pressure Field: Role of fluctuating pressure in structure failure

The stability of concrete slabs or rock blocks at plunge pool bottom of spillways (or at joints locations for segmented culverts) depends on the instantaneous pressure field

This pressure propagates under lining elements through concrete fissures, open or failed joints, can generate an uplift force that can lead to dislodging of the lining.

Oroville Reservoir

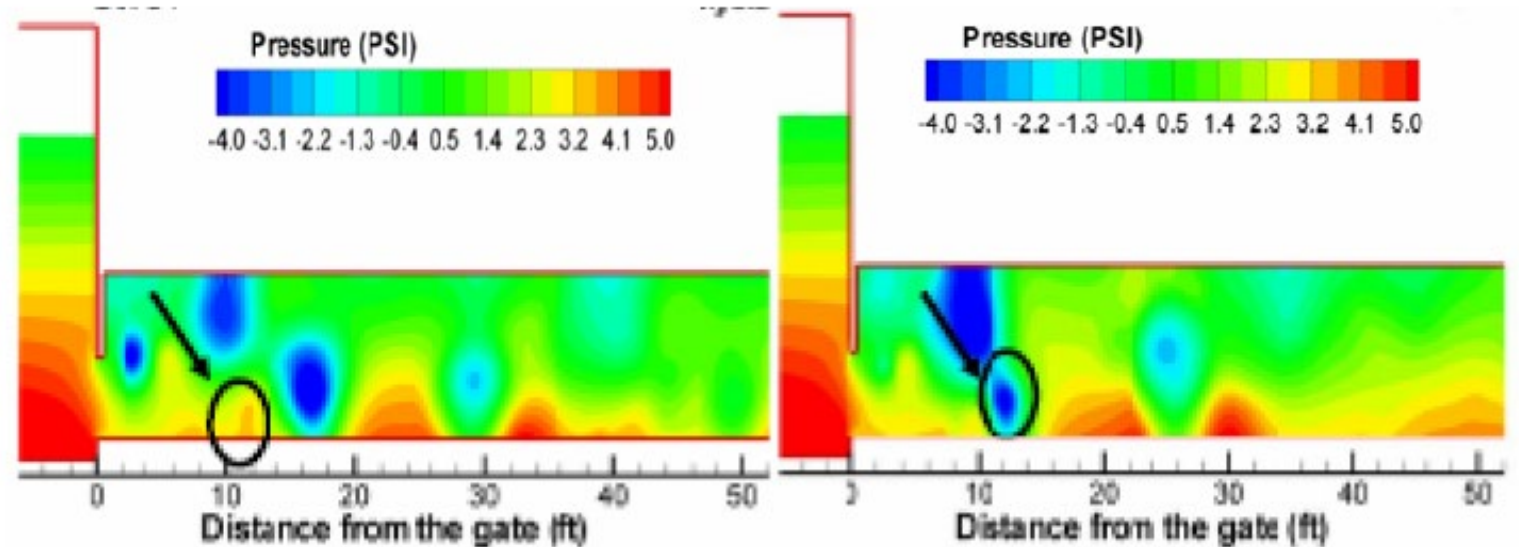


California Department of Water Resources photos taken Tuesday, Feb. 7, 2017, show extensive damage to the Oroville Dam spillway and severe erosion to area adjacent to spillway structure. (Kelly M. Grow/Department of Water Resources)



A concrete section eroded on the middle section of the spillway. (Kelly M. Grow/California Department of Water Resources)

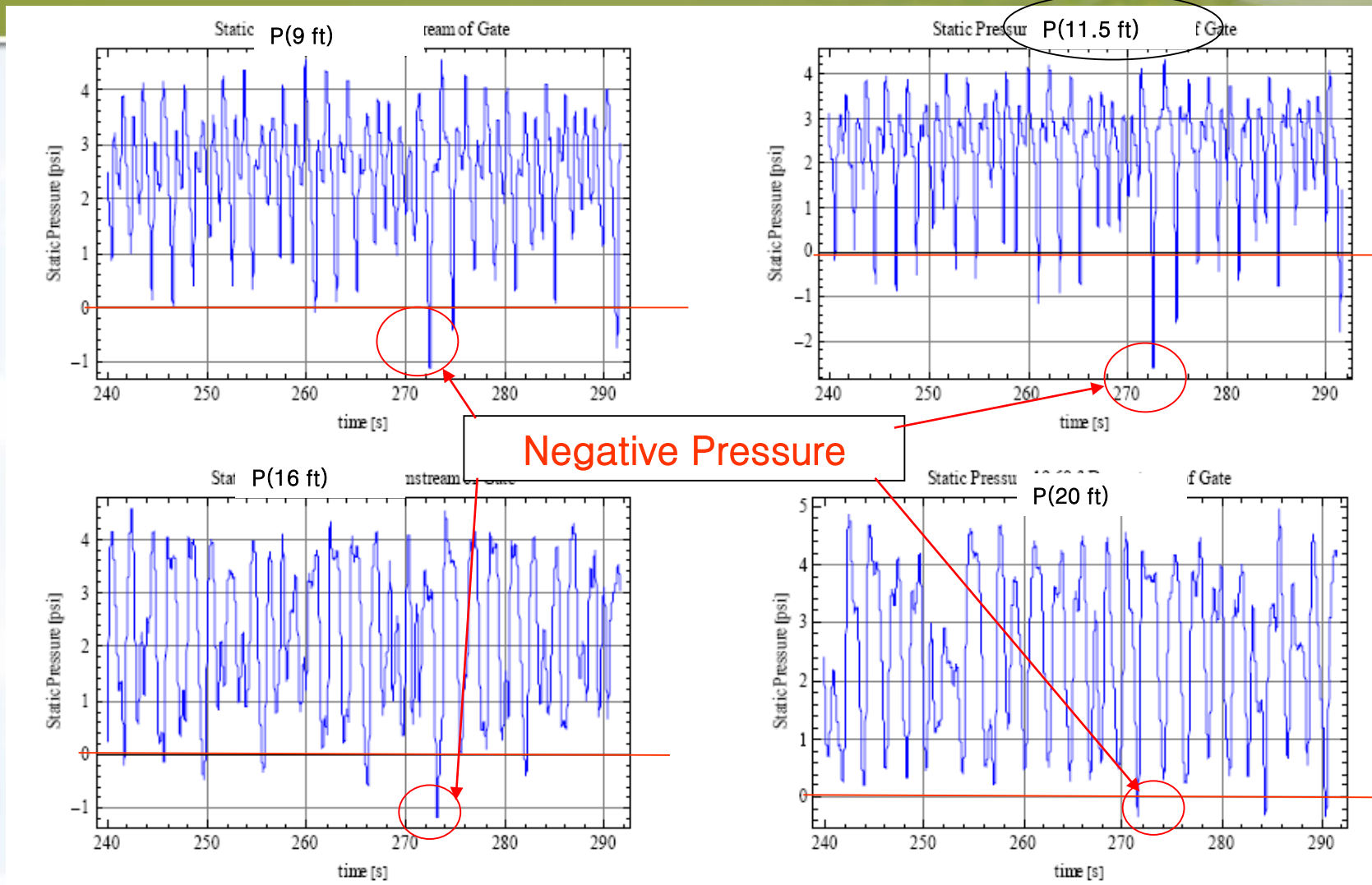
Dynamic Pressure Field Analysis with DES model for S-375



Pressure fields in the culvert barrel at the times when the minimum (right) and maximum (Left) fluctuations occurred at a point 11 feet from the Gate

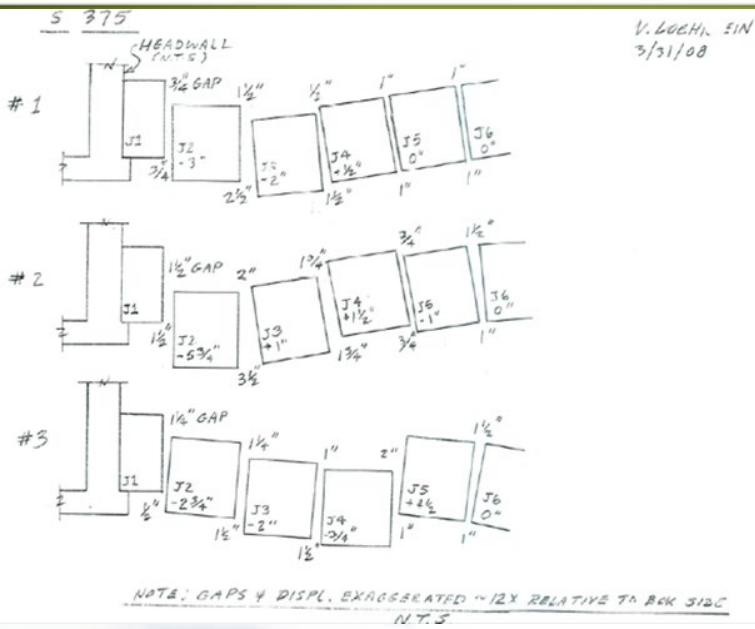
J. Zeng et al. (2009)

Pressure Fluctuations at Selected Locations

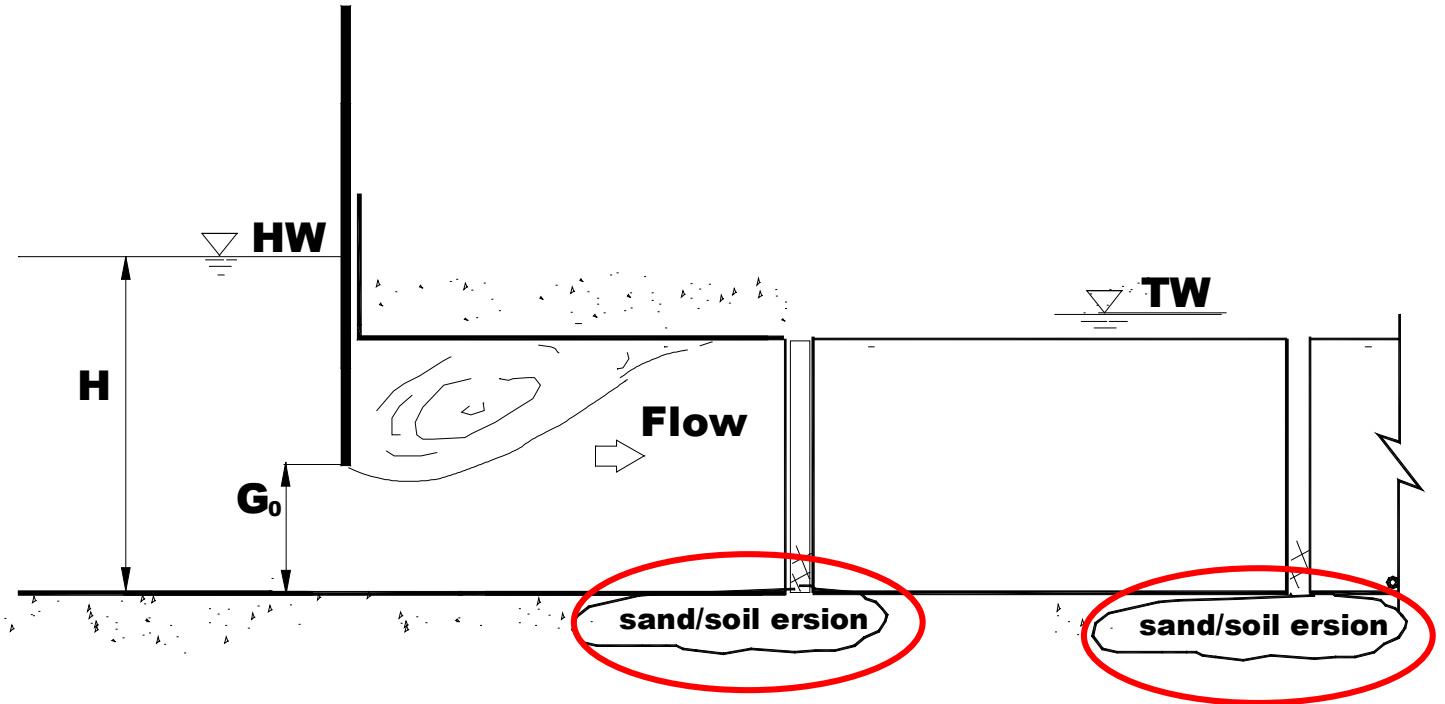


J. Zeng et al. (2009)

Pressures fluctuations can reach 11 psi



Damage analysis—Sand/soil erosion



The seepage and inner negative pressure lead to the loss of the sand/soil foundation after the joint failure.

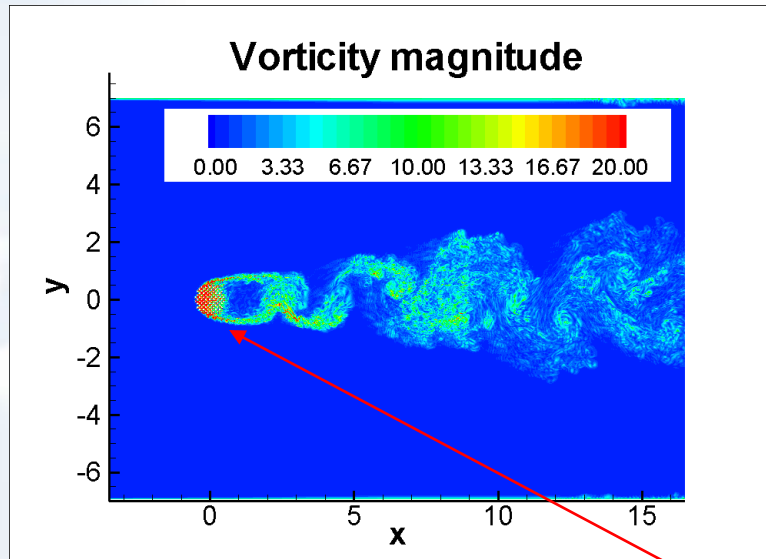
J. Zeng et al. (2009)



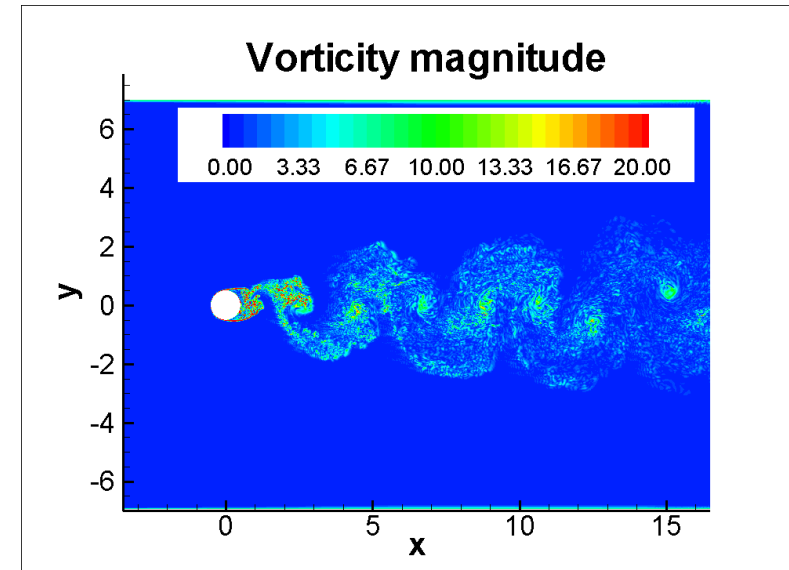
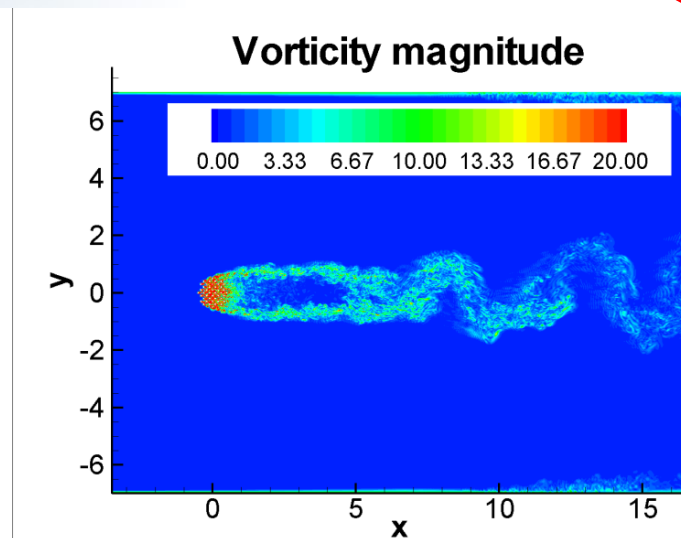
FLOW PAST A CIRCULAR PATCH OF VEGETATION

-Von Karman vortex street is qualitatively similar to that observed past solid cylinders

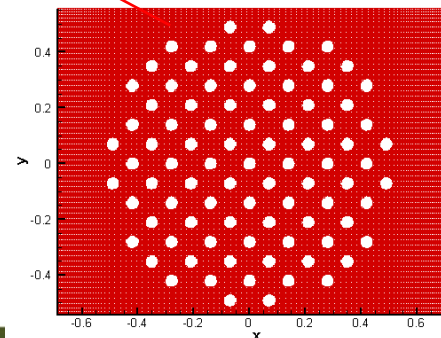
SVF=20%
(89 cylinders)



SVF=10%
(44 cylinders)



SVF=100% (solid cylinder)



Chang & Tsai (NHPC, Taiwan)
G. Constantinescu (Univ. of Iowa)



(RiverFlow 2016)

Flow past a surface-mounted porous cylinder

-MIT experiment for an emergent patch (Nepf, 2012)

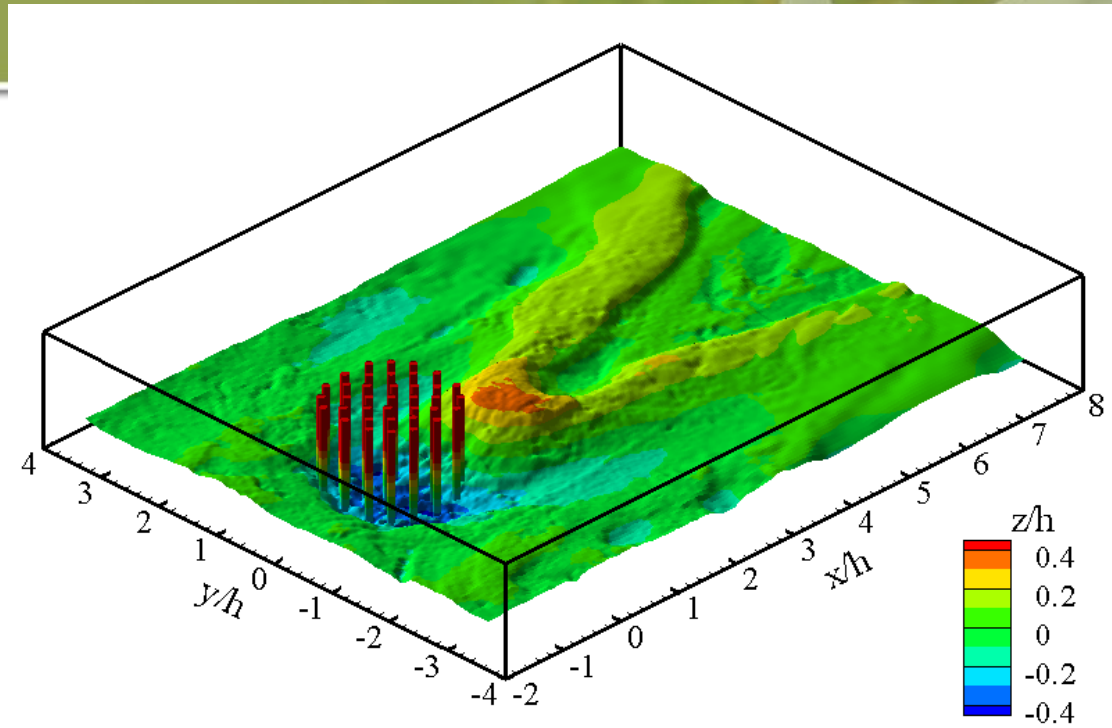
-Solid Volume Fraction (SVF)=13%

-37 cylinders

- $H/D=0.55$, $d/D=0.06$ $H=0.12$ m

- $Re=30,000$ $Re_D=60,000$

-Flat bed & Equilibrium scour



Main questions:

-What drives scour within and around the patch?

-Do necklace vortices form and do they play an important role in development of scour hole?

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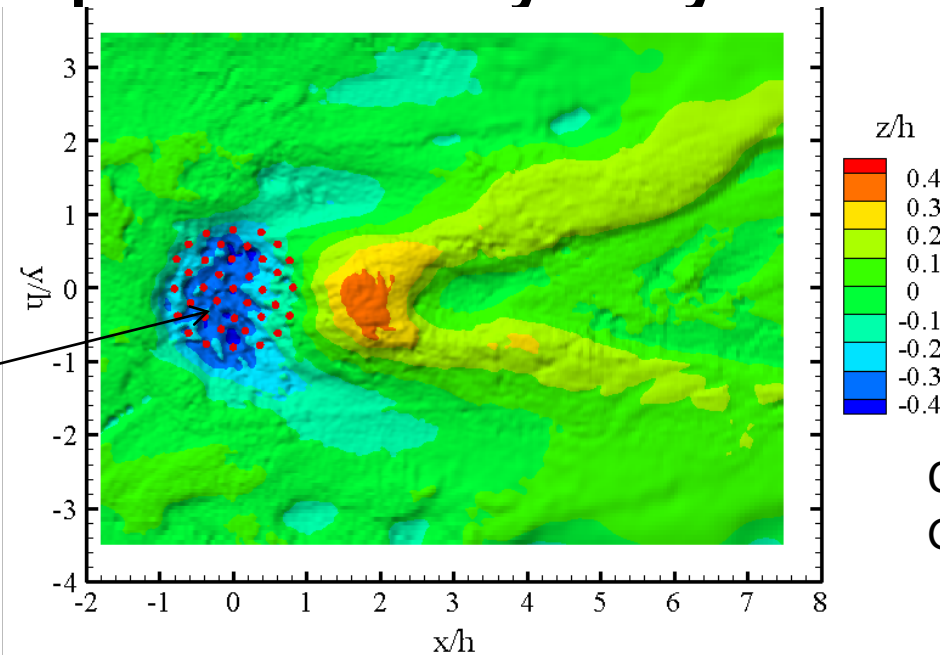
(RiverFlow 2016)

Flow past a surface-mounted porous cylinder

Equilibrium bathymetry

Maximum scour occurs
inside porous cylinder

No significant scour in
front of porous cylinder

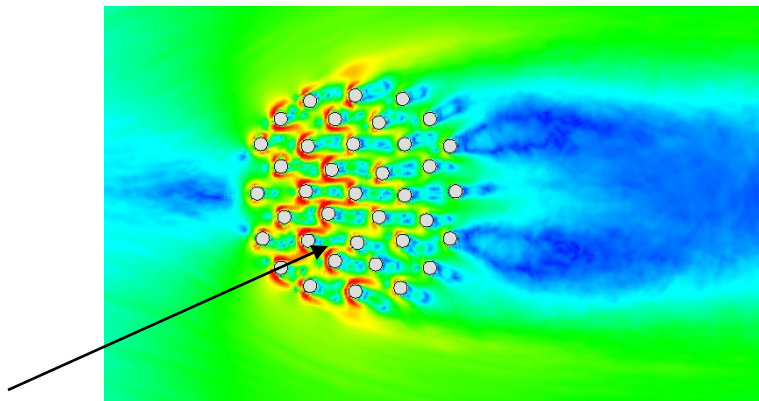


SVF=13%

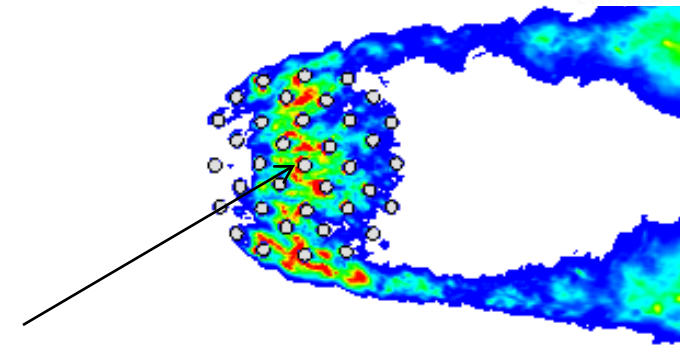
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(RiverFlow 2016)



Bed friction velocity (FB)



Mean Bed friction vel Fluctuations at the bed (FB)

CONCLUSIONS AND SUMMARY

CFD can be a powerful tool in addressing emerging restoration hydraulics issues, including:

- Hydraulic downpull on spillway gates - **K-epsilon or K-Omega models**
- Depth and extent of scour downstream of hydraulic structures
Implement Empirical/semi-empirical sediment transport equations in a CFD solver
Large Eddies Simulations (LES)
- Role of pressure fluctuations in structure failure - **Detached Eddies Simulations (DES)**
- 3D simulations of vegetated flows
Detached Eddies Simulations (DES)